

S1.0

FRAMING NOTES:

- [illegible]

7. These plans should be checked and used in their entirety as information vital to the design of construction. Information may appear on more than one sheet.
8. Contractor assumes all liability for any deviation from the plan not addressed by the final construction.
9. All drawings should be read or calculated but not re-created.
10. Verify all conditions and dimensions at job before beginning construction.
11. Windows and Doors
 - a. Windows shown as required egress windows shall meet local building codes.
 - b. All glazing in walls, doors and egress windows shall be tempered unless otherwise noted.
 - c. Thresholds are approximate and may vary according to manufacturers.
 - d. Unless noted otherwise, maximum heights are nominal and shall be noted as such.

1. *Chrysomelids* (see also 1.1.1)

- | | | | |
|-----------------|---------|------------|----------|
| Hot Deck Load: | -20 PSF | Draw Load: | -20 PSF |
| Roof Dead Load: | -10 PSF | Wind Load: | -110 MPH |
- 2 - Basing for footings on undisturbed soil at 95% compacted fill is assumed to be 2,000 P.S.F. u.s.s.
Verify soil-bearing capacity and any underlying conditions.
- 3 - All designs are in accordance with State Building Codes.
- 4 - Any deviation from the structural requirements of these plans without consulting with the general contractor is prohibited and thereby releases the general contractor from any responsibility.

STRUCTURAL DESIGN

Importance Factors	Score	$\phi_{\text{Importance}}$	1.0
	Score	$\phi_{\text{Importance}}$	1.0
User Loads	Self	70	psf
	Nonuniform	10	psf
	Wind	100	psf
Ground Snow Load		20	psf
Wind Load	Basic Wind Speed	115	mph (KZC B)

DESIGN CATEGORY ☐ A ☐ B ☒ C ☐ D

Provide the following Scenario Design Parameters:

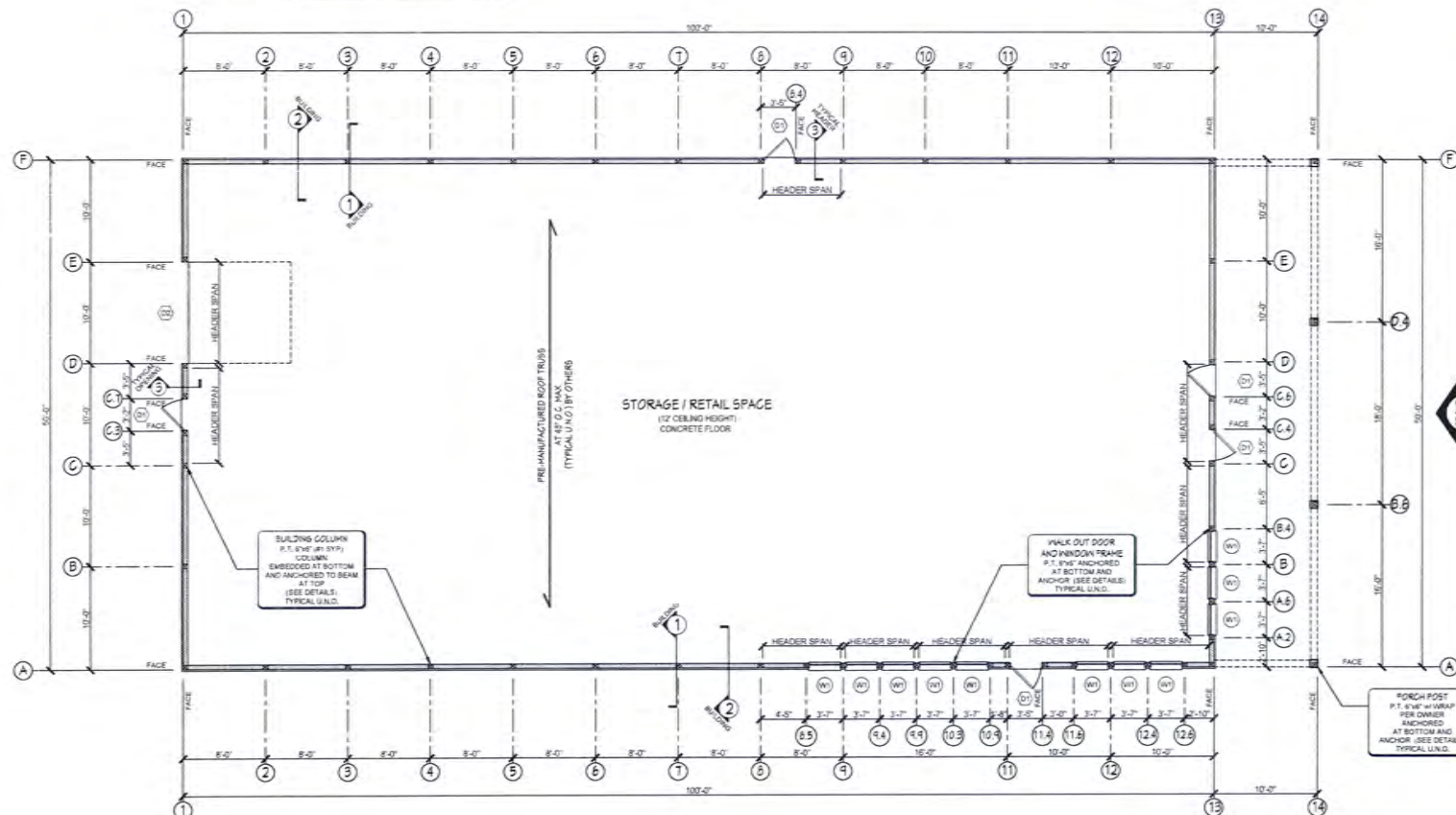
Risk Category/Seismicity: ☐ A ☐ B ☐ C ☐ D ☐ E ☐ F ☐ G ☐ H ☐ I ☐ J ☐ K ☐ L ☐ M ☐ N ☐ O ☐ P ☐ Q ☐ R ☐ S ☐ T ☐ U ☐ V ☐ W ☐ X ☐ Y ☐ Z

Seismic Response Acceleration: ☐ 0.1 ☐ 0.2 ☐ 0.3 ☐ 0.4 ☐ 0.5 ☐ 0.6 ☐ 0.7 ☐ 0.8 ☐ 0.9 ☐ 1.0 ☐ 1.1 ☐ 1.2 ☐ 1.3 ☐ 1.4 ☐ 1.5 ☐ 1.6 ☐ 1.7 ☐ 1.8 ☐ 1.9 ☐ 2.0 ☐ 2.1 ☐ 2.2 ☐ 2.3 ☐ 2.4 ☐ 2.5 ☐ 2.6 ☐ 2.7 ☐ 2.8 ☐ 2.9 ☐ 3.0 ☐ 3.1 ☐ 3.2 ☐ 3.3 ☐ 3.4 ☐ 3.5 ☐ 3.6 ☐ 3.7 ☐ 3.8 ☐ 3.9 ☐ 4.0 ☐ 4.1 ☐ 4.2 ☐ 4.3 ☐ 4.4 ☐ 4.5 ☐ 4.6 ☐ 4.7 ☐ 4.8 ☐ 4.9 ☐ 5.0 ☐ 5.1 ☐ 5.2 ☐ 5.3 ☐ 5.4 ☐ 5.5 ☐ 5.6 ☐ 5.7 ☐ 5.8 ☐ 5.9 ☐ 6.0 ☐ 6.1 ☐ 6.2 ☐ 6.3 ☐ 6.4 ☐ 6.5 ☐ 6.6 ☐ 6.7 ☐ 6.8 ☐ 6.9 ☐ 7.0 ☐ 7.1 ☐ 7.2 ☐ 7.3 ☐ 7.4 ☐ 7.5 ☐ 7.6 ☐ 7.7 ☐ 7.8 ☐ 7.9 ☐ 8.0 ☐ 8.1 ☐ 8.2 ☐ 8.3 ☐ 8.4 ☐ 8.5 ☐ 8.6 ☐ 8.7 ☐ 8.8 ☐ 8.9 ☐ 9.0 ☐ 9.1 ☐ 9.2 ☐ 9.3 ☐ 9.4 ☐ 9.5 ☐ 9.6 ☐ 9.7 ☐ 9.8 ☐ 9.9 ☐ 10.0 ☐ 10.1 ☐ 10.2 ☐ 10.3 ☐ 10.4 ☐ 10.5 ☐ 10.6 ☐ 10.7 ☐ 10.8 ☐ 10.9 ☐ 11.0 ☐ 11.1 ☐ 11.2 ☐ 11.3 ☐ 11.4 ☐ 11.5 ☐ 11.6 ☐ 11.7 ☐ 11.8 ☐ 11.9 ☐ 12.0 ☐ 12.1 ☐ 12.2 ☐ 12.3 ☐ 12.4 ☐ 12.5 ☐ 12.6 ☐ 12.7 ☐ 12.8 ☐ 12.9 ☐ 13.0 ☐ 13.1 ☐ 13.2 ☐ 13.3 ☐ 13.4 ☐ 13.5 ☐ 13.6 ☐ 13.7 ☐ 13.8 ☐ 13.9 ☐ 14.0 ☐ 14.1 ☐ 14.2 ☐ 14.3 ☐ 14.4 ☐ 14.5 ☐ 14.6 ☐ 14.7 ☐ 14.8 ☐ 14.9 ☐ 15.0 ☐ 15.1 ☐ 15.2 ☐ 15.3 ☐ 15.4 ☐ 15.5 ☐ 15.6 ☐ 15.7 ☐ 15.8 ☐ 15.9 ☐ 16.0 ☐ 16.1 ☐ 16.2 ☐ 16.3 ☐ 16.4 ☐ 16.5 ☐ 16.6 ☐ 16.7 ☐ 16.8 ☐ 16.9 ☐ 17.0 ☐ 17.1 ☐ 17.2 ☐ 17.3 ☐ 17.4 ☐ 17.5 ☐ 17.6 ☐ 17.7 ☐ 17.8 ☐ 17.9 ☐ 18.0 ☐ 18.1 ☐ 18.2 ☐ 18.3 ☐ 18.4 ☐ 18.5 ☐ 18.6 ☐ 18.7 ☐ 18.8 ☐ 18.9 ☐ 19.0 ☐ 19.1 ☐ 19.2 ☐ 19.3 ☐ 19.4 ☐ 19.5 ☐ 19.6 ☐ 19.7 ☐ 19.8 ☐ 19.9 ☐ 20.0 ☐ 20.1 ☐ 20.2 ☐ 20.3 ☐ 20.4 ☐ 20.5 ☐ 20.6 ☐ 20.7 ☐ 20.8 ☐ 20.9 ☐ 21.0 ☐ 21.1 ☐ 21.2 ☐ 21.3 ☐ 21.4 ☐ 21.5 ☐ 21.6 ☐ 21.7 ☐ 21.8 ☐ 21.9 ☐ 22.0 ☐ 22.1 ☐ 22.2 ☐ 22.3 ☐ 22.4 ☐ 22.5 ☐ 22.6 ☐ 22.7 ☐ 22.8 ☐ 22.9 ☐ 23.0 ☐ 23.1 ☐ 23.2 ☐ 23.3 ☐ 23.4 ☐ 23.5 ☐ 23.6 ☐ 23.7 ☐ 23.8 ☐ 23.9 ☐ 24.0 ☐ 24.1 ☐ 24.2 ☐ 24.3 ☐ 24.4 ☐ 24.5 ☐ 24.6 ☐ 24.7 ☐ 24.8 ☐ 24.9 ☐ 25.0 ☐ 25.1 ☐ 25.2 ☐ 25.3 ☐ 25.4 ☐ 25.5 ☐ 25.6 ☐ 25.7 ☐ 25.8 ☐ 25.9 ☐ 26.0 ☐ 26.1 ☐ 26.2 ☐ 26.3 ☐ 26.4 ☐ 26.5 ☐ 26.6 ☐ 26.7 ☐ 26.8 ☐ 26.9 ☐ 27.0 ☐ 27.1 ☐ 27.2 ☐ 27.3 ☐ 27.4 ☐ 27.5 ☐ 27.6 ☐ 27.7 ☐ 27.8 ☐ 27.9 ☐ 28.0 ☐ 28.1 ☐ 28.2 ☐ 28.3 ☐ 28.4 ☐ 28.5 ☐ 28.6 ☐ 28.7 ☐ 28.8 ☐ 28.9 ☐ 29.0 ☐ 29.1 ☐ 29.2 ☐ 29.3 ☐ 29.4 ☐ 29.5 ☐ 29.6 ☐ 29.7 ☐ 29.8 ☐ 29.9 ☐ 30.0 ☐ 30.1 ☐ 30.2 ☐ 30.3 ☐ 30.4 ☐ 30.5 ☐ 30.6 ☐ 30.7 ☐ 30.8 ☐ 30.9 ☐ 31.0 ☐ 31.1 ☐ 31.2 ☐ 31.3 ☐ 31.4 ☐ 31.5 ☐ 31.6 ☐ 31.7 ☐ 31.8 ☐ 31.9 ☐ 32.0 ☐ 32.1 ☐ 32.2 ☐ 32.3 ☐ 32.4 ☐ 32.5 ☐ 32.6 ☐ 32.7 ☐ 32.8 ☐ 32.9 ☐ 33.0 ☐ 33.1 ☐ 33.2 ☐ 33.3 ☐ 33.4 ☐ 33.5 ☐ 33.6 ☐ 33.7 ☐ 33.8 ☐ 33.9 ☐ 34.0 ☐ 34.1 ☐ 34.2 ☐ 34.3 ☐ 34.4 ☐ 34.5 ☐ 34.6 ☐ 34.7 ☐ 34.8 ☐ 34.9 ☐ 35.0 ☐ 35.1 ☐ 35.2 ☐ 35.3 ☐ 35.4 ☐ 35.5 ☐ 35.6 ☐ 35.7 ☐ 35.8 ☐ 35.9 ☐ 36.0 ☐ 36.1 ☐ 36

LATERAL DESIGN CONTROL: Earthquake ☐ Wind ☒

SOIL BEARING CAPACITIES:
Field Test (provide copy of test report) _____
Presumptive Bearing capacity _____ 2,000

ROOF TRUSS LAYOUT NOTE
 ROOF PLAN IS SCHEMATIC ONLY. ACTUAL
 TRUSS DESIGN, LAYOUT AND LOCATIONS
 SHALL BE DETERMINED BY TRUSS
 MANUFACTURER.
 ANY HEADER BEARING POINT LOAD FROM
 GIRDER TRUSS SHALL BE VERIFIED BY TRUSS
 MANUFACTURER.
 SUBMIT DRAWINGS TO DESIGNER FOR
 APPROVAL PRIOR TO CONSTRUCTION.



ISSUED FOR
CONSTRUCTION



DESIGNER
Phone: (704)-253-8080
eMail: DTHomeDesigns@GMail.Com

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Phone: (704) 819-6415
email: Scott@Solidrockstructural.com

HEATED	4500 S.F.
PORCH	500 S.F.
Total Under Rent - 5000 S.F.	

[illegible]

Drawn By: DRG	Issue Date: 12.23.19
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A POLE BARN FOR
CAHILLS LILLY PAD INC

319 T B Stanley Hwy.
Bassett, VA 24055

Plan Name/Number:

2019-195

Sheet Title:

**STRUCTURAL
FLOOR PLAN**

SCHEMATIC

Sheet No.

S3.0

1. Concrete shall be a minimum of 2070 g/cm³ in 28 days, unless otherwise stated.
2. Concrete strength by maximum shall be a minimum of 50 MPa in 28 days with 4.0% to 6% air - maximum in excess of 6.0%.
3. Freshness is NOT a test item for wettable work such as pouring, tamping or spalling.
4. 90 day strength shall be a minimum of 42 MPa, unless otherwise stated.
5. For retaining walls without the use of special drainage or weirs.
6. Water and moisture protection:
 - a. In above basements waterproofing when exterior grade is at or above ground level a basement shall be constructed of waterproofed plastic foundation exterior and a slope is allowed any dry and/or natural drainage surfaces.
 - b. In all building exterior surface penetrations with potential exposure to water shall be for higher exterior grade and/or grade, and/or any other waterproofing or drainage system or waterproofing testings.
7. All work in earth will be performed to a minimum depth for concrete tested.

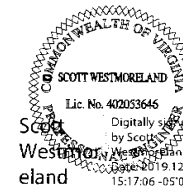


D&T
HOME DESIGN

DESIGNER
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Phone (704) 819-6415
email Scott@SolidRocksStructural.com

HEATED	4500 S.
PORCH	500 S.
Total Under Sock	5000 S.

[illegible]

Drawn By DRG	Issue Date 12.23.19
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4 POLE BARN FOR
CAHILLS LILLY PAD INC.

319 T B Stanley Hwy.
Bassett, VA 24055

Plan Name/Number:

2019-195

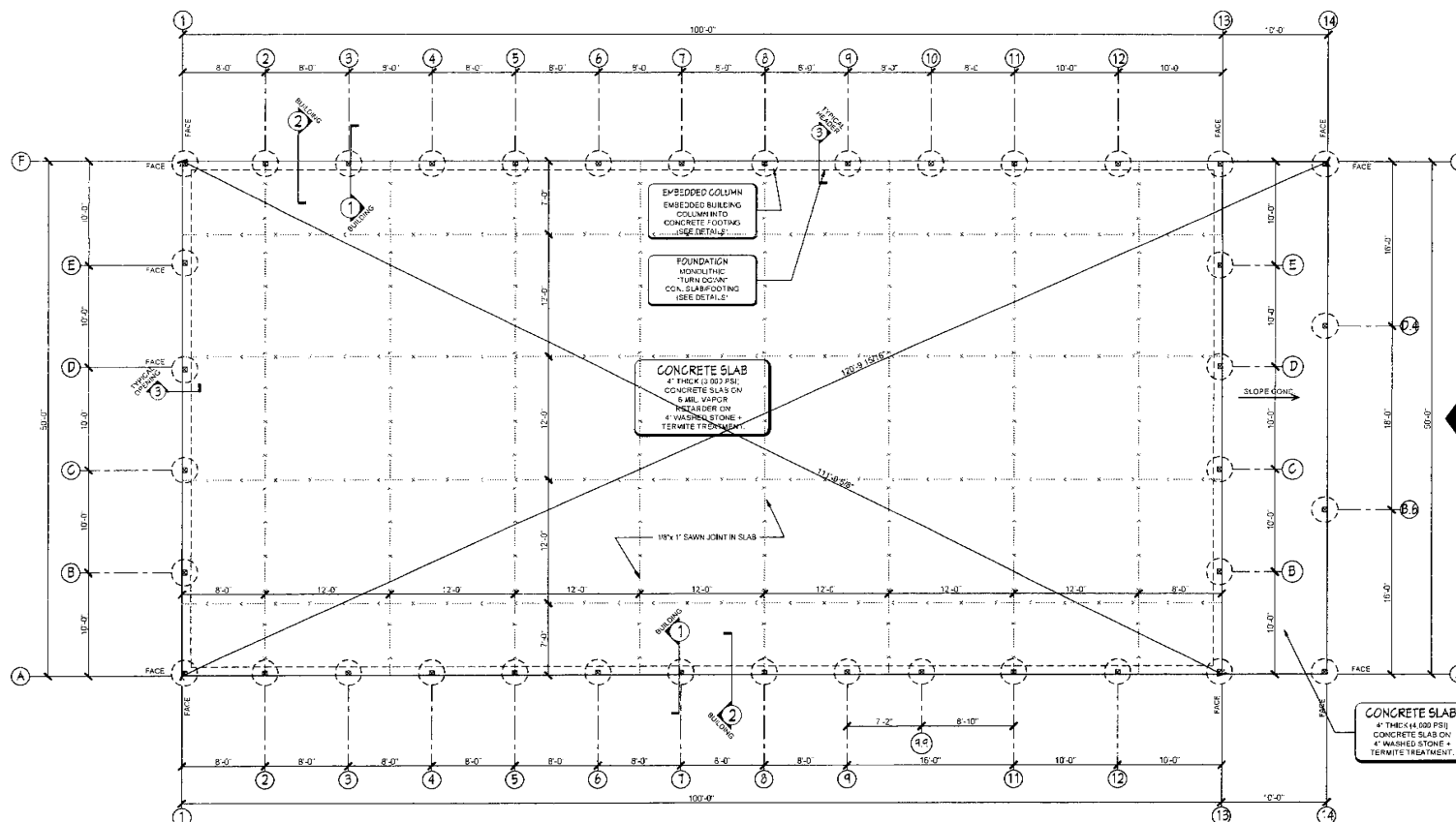
Sheet Title

FOUNDATION PLAN

SCALE 1/8" = 1'-0"

Steel No.

S2.0



ISSUED FOR
CONSTRUCTION



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SQUARE FOOTAGE

HEATED	4500 S.F.
PORCH	500 S.F.
Total Under Roof - 5000 S.F.	

REVISIONS

No.	Date	By	Description

Drawn By: DRG Issue Date: 12.23.19

A PCLE BARN FOR
CAHILLS LILLY PAD INC.

319 T B Stanley Hwy.
Bassett, VA 24055

Plan Name/Number
2019-195

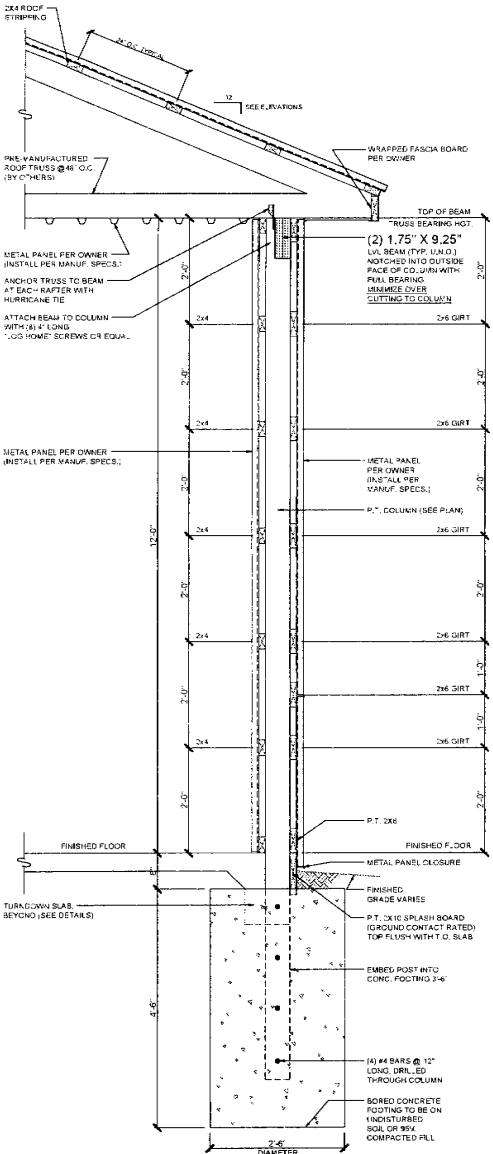
Sheet Title

SECTIONS & DETAILS

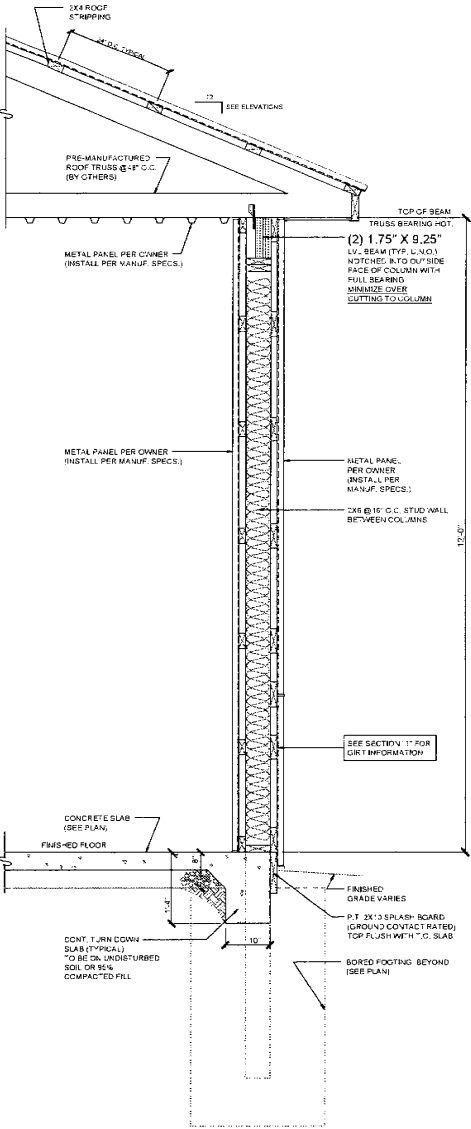
SCALE AS NOTED

Sheet No.

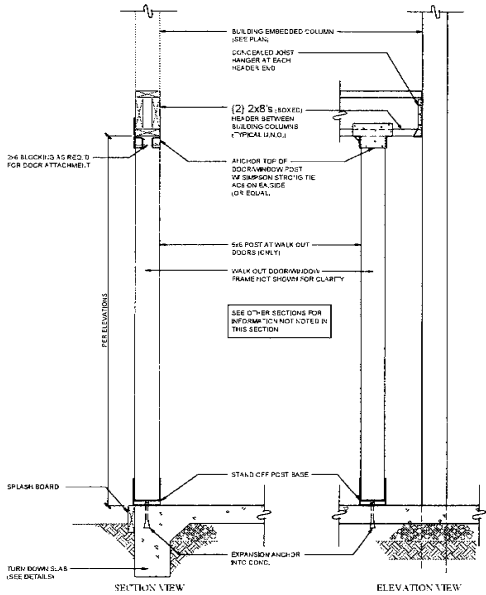
S4.0



1 TYPICAL WALL SECTION
AT EMBEDDED COLUMN LOCATIONS



2 TYPICAL WALL SECTION
AT STUD WALL LOCATIONS



3 DOOR/WINDOW FRAME DETAIL



JL Truss
 2556 Walkers Well Rd
 Chatham, Virginia 24531
 PH. 1-434-432-0340 FX. 1-434-432-0363

Assembly by Group with Plates

Page 1 of 2
 JLT-0049 - JLT-0049 Chatham Amish Job

Jesse Cahill

Truss: T1

Spacing: 4-0-0

Group: 1

Span	Pitch	Qty	LOH	ROH	Plies	Weight	BDFT	Table
53-0-0	4.00 / 4.00	29	0-0-0	0-0-0	1	397.68	176.67	

27 regulars

1 gable with 2 horizontal webs and 2x4 spruce under top cord.
 1 gable with 24" on center vertical webs

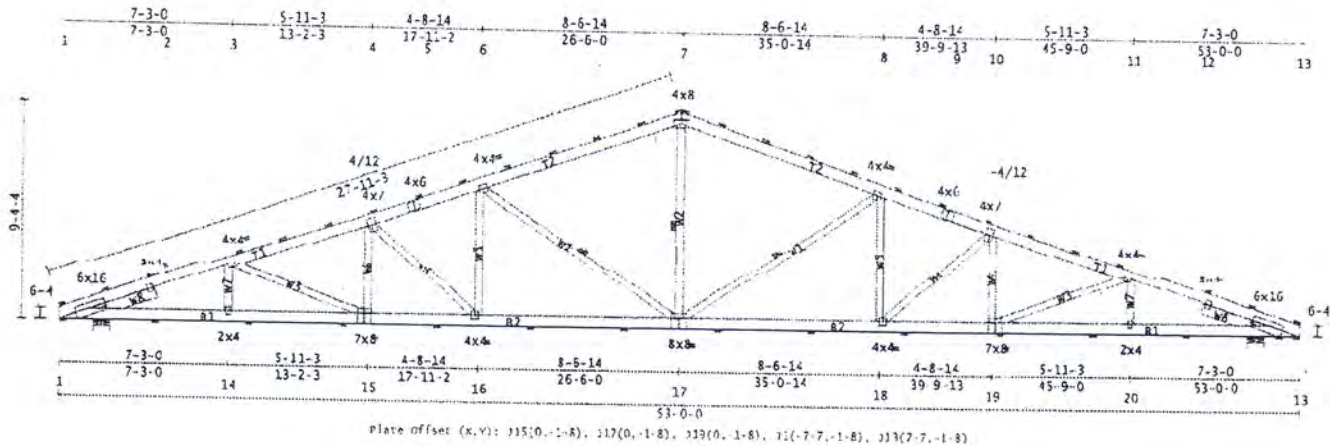


Plate offset (x,v): 115(0,-1-8), 117(0,-1-8), 119(0,-1-8), 121(-7-7,-1-8), 113(7-7,-1-8)

Overall Truss Height 9-4-4		Loads: TCLL: 20 TCDI: 5 BCLL: 0 BCDL: 5 GSL: 20		Overhang Rake Distance L/R 0-0-0 / 0-0-0		Total BDFT: 176.67 Bottom Chord: 56.00 Sliders: 0.00 Top Chord: 60.00 Web: 61.67		Extended BDFT: 5,123.33 Bottom Chord: 1,624.00 Sliders: 174.00 Top Chord: 1,740.00 Web: 1,695.23	
Plates Size: Qty:		2x4 116	3x4 116	4x4 348	4x6 116	4x8 58	4x10 58	4x12 58	Total Sq In: 37120
T1-T1		15-10-3 15-11-1 16-0-0		18.4 / 90.0 11-9-5 12-5-9 13-0-7		Scarf: 1-3-15 V: 0-5-8 Butt: 0-0-7 PAH: 0-0-7		5-1 0-7	
T1-B1 PC: 13-0-7		13-5-9		12-1-1 12-0-2 11-11-3		9-9-8 10-0-0 9-8-4		52.5 / 55.9 55.9 / 34.1	
T1-B2 PC: 13-5-9		8-4-6 8-4-15 8-4-6		5-9-3 6-0-0 5-6-9		5-9-7 6-0-0 5-8-7		60.5 / 47.9 47.9 / 42.1	
T1-W2		8-4-6 8-4-15 8-4-6		5-9-3 6-0-0 5-6-9		5-9-7 6-0-0 5-8-7		60.5 / 47.9 47.9 / 42.1	
T1-W3		8-4-6 8-4-15 8-4-6		5-9-3 6-0-0 5-6-9		5-9-7 6-0-0 5-8-7		60.5 / 47.9 47.9 / 42.1	
T1-W4		8-4-6 8-4-15 8-4-6		5-9-3 6-0-0 5-6-9		5-9-7 6-0-0 5-8-7		60.5 / 47.9 47.9 / 42.1	





JL Truss
 2556 Walkers Well Rd
 Chatham, Virginia 24531
 PH. 1-434-432-0340 FX. 1-434-432-0363

Assembly by Group with Plates

Page 2 of 2
 JLT-0049 - JLT-0049 Chatham Amish Job

T1 -W5 OL: 5-7-4 71.6 \ 5-7-4 90.0 5-6-10 5-6-1 (58)SP #1 2x4 PICK: 3-0-0 Timbermill	T1 -W6 OL: 4-0-4 71.6 \ 4-0-4 90.0 3-11-11 3-11-2 (58)SP #1 2x4 PICK: 5-0-0 Timbermill	T1 -W7 OL: 2-0-9 71.6 \ 2-0-9 90.0 1-11-15 1-11-6 (58)SP #1 2x4 PICK: 3-0-0 Timbermill
Group: 1 T1 -W8 OL: 3-0-0 18.4 \ 3-0-0 90.0 2-3-12 1-7-8 (58)SP #1 2x6 PICK: 3-0-0 Timbermill		

FAX COVER SHEET

TO

COMPANY

FAX NUMBER 14344320363

FROM Chad Roop

DATE 10/17/2019 09:58:47 EDT

RE Seals for Chatham Amish Job

COVER MESSAGE

Jason,

These are the sealed drawings for this truss.

Jamie Brinegar | Simpson Strong-Tie | Truss Specialist -- Northeast |
Cell: 443.289.0780 | Help Desk: 866.252.8606 | <http://www.strongtie.com>

**TRUSS DESIGN DRAWINGS & DETAILS****Simpson Strong-Tie
Company, Inc.**5956 W. Las Positas Blvd.
Pleasanton, CA 94588
(800) 999-5099
www.strongtie.com

Prepared for: J-L Truss
Job: JLT-0049 - JLT-0049 Chatham Amish Job
Date: 10/16/2019 12:56 PM
Ref Number: 100212

Notes:

1. The truss design drawings referenced below have been prepared based on design criteria and requirements set forth in the Construction Documents, as communicated by the Truss Manufacturer.
2. The engineer's signature on these drawings indicates professional engineering responsibility solely for the individual components to be able to resist the design loads indicated, utilizing all the design parameter and materials indicated or referenced on each individual design.
3. It is the Building Designer's responsibility to review the truss design drawings to insure compatibility with the Building design. Refer to all notes on the individual truss design drawings.

1 Truss Design Drawing(s):

1-T1: SID 602320



Component Solutions™ Important Information & General Notes

General Notes

- Each Truss Design Drawing (TDD) provided with this sheet has been prepared in conformance with ANSI/TPI 1. Refer to ANSI/TPI 1 Chapter 2 for the responsibilities of all parties involved, which include but are not limited to the responsibilities listed on this sheet, and for the definitions of all capitalized terms referenced in this document.
- TDDs should not be assumed to be to scale.
- The Contractor and Building Designer shall review and approve the Truss Submittal Package.
- The suitability and use of the component depicted on the TDD for any particular building design is the responsibility of the Building Designer.
- The Building Designer is responsible for the anchorage of the truss at all bearing locations as required to resist uplift, gravity and lateral loads, and for all Truss-to-Structural Element connections except Truss-to-Truss connections.
- The Building Designer shall ensure that the supporting structure can accommodate the vertical and/or horizontal truss deflections.
- Unless specifically stated otherwise, each Design assumes trusses will be adequately protected from the environment and will not be used in corrosive environments unless protected using an approved method. This includes not being used in locations where the sustained temperature is greater than 150°F.
- Trusses are designed to carry loads within their plane. Any out-of-plane loads must be resisted by the Permanent Building Stability Bracing.
- Design dead loads must account for all materials, including self-weight. The TDD notes will indicate the min. pitch above which the dead loads are automatically increased for pitch effects.
- Trusses installed with roof slopes less than 0.25/12 may experience (but are not designed for) ponding. The Building Designer must ensure that adequate drainage is provided to prevent ponding.
- Camber is a non-structural consideration and is the responsibility of truss fabricator.

Handling, Installing, Restraint & Bracing

- The Contractor is responsible for the proper handling, erection, restraint and bracing of the Trusses. In lieu of job-specific details, refer to BCSI.
- ANSI/TPI 1 stipulates that for trusses spanning 60' or greater, the Owner shall contract with any Registered Design Professional for the design and inspection of the temporary and permanent truss restraint and bracing. Simpson Strong-Tie is not responsible for providing these services.
- Trusses require permanent lateral restraint to be applied to chords and certain web members (when indicated) at the locations or intervals indicated on the TDD. Web restraints are to be located at mid points, or third points of the member and chord purlins are not to exceed the spacing specified by the TDD. Chords shown without bracing indicated are assumed to be continuously braced by sheathing or drywall. Permanent lateral restraint shall be accomplished in accordance with: standard industry lateral restraint/bracing details in BCSI-B3 or BCSI-B7, supplemental bracing details referenced on the TDD, or as specified in a project-specific truss permanent bracing plan provided by the Building Designer.
- Additional building stability permanent bracing shall be installed as specified in the Construction Documents.
- Special end wall bracing design considerations may be required if a flat gable end frame is used with adjacent trusses that have sloped bottom chords (see BCSI-B3).
- Do not cut, drill, trim, or otherwise alter truss members or plates without prior written approval of an engineer, unless specifically noted on the TDD.
- Piggyback assemblies shall be braced as per BCSI-B3 unless otherwise specified in the Construction Documents.
- For floor trusses, when specified, Strongbacking shall be installed per BCSI-B7 unless otherwise specified in the Construction Documents.

Referenced Standards

ANSI/TPI 1: National Design Standard for Metal Plate Connected Wood Truss Construction, a Truss Plate Institute publication (www.tpinst.org).

BCSI: Guide to Good Practice for Handling, Installing, Restraining & Bracing Metal Plate Connected Wood Trusses, a joint publication of the Truss Plate Institute (www.tpinst.org) and the Structural Building Components Association (www.sbcindustry.com).

Symbols and Nomenclature

5x7 Plate size; the first digit is the plate width (perp. to the slots) and the second digit is the plate length (parallel to the slots).

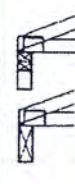
5x7-18 -18, -18S5, or -18S6 following the plate size indicates different 18 gauge plate types.

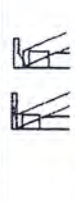
 These symbols following the plate size indicate the direction of the plate length (and tooth slots) for square and nearly square plates.

10-3-14 Dimensions are shown in feet-inches-sixteenths (for this example, the dimension is 10'-3 14/16").

 Joints are numbered left to right, first along the top chord and then along the bottom chord. Mid-panel splice joint numbers are not shown on the drawing. Members are identified using their end joint numbers (e.g., TC 2-3).

 When this symbol is shown, permanent lateral restraint is required. Lateral restraint may be applied to either edge of the member. See Note 3 under Handling, Installing, Restraint & Bracing for more information.

 Bearing supports (wall, beam, etc.), locations at which the truss is required to have full bearing. Minimum required bearing width for the given reactions are reported on the TDD. Required bearing widths are based on the truss material and indicated PSI of the support material. The Building Designer is responsible for verifying that the capacity of the support material exceeds the indicated PSI, and for all other bearing design considerations.

 Truss-to-Truss or Truss-to-Structural Element connection, which require a hanger or other structural connection (e.g., toe-nail) that has adequate capacity to resist the maximum reactions specified in the Reaction Summary. Structural connection type is not limited by type shown on TDD. Toe-nails may be used where hanger type shown where allowed by detail or other connection design information. Design of the Structural Element and the connection of the Truss to a Structural Element is by others.

Note: These symbols are for graphical interpretation only; they are not intended to give any indication of the geometry requirements of the actual item that is represented.

Materials and Fabrication

- Design assumes truss is manufactured in accordance with the TDD and the quality criteria in ANSI/TPI 1 Chapter 3, unless more restrictive criteria are part of the contract specifications.
- Unless specifically stated, lumber shall not exceed 19% moisture content at time of fabrication or in service.
- Design is not applicable for use with fire retardant, preservative treated or green lumber unless specifically stated on the TDD.
- Plate type, size, orientation and location indicated are based on the specified design parameters. Larger plate sizes may be substituted in accordance with ANSI/TPI, Section 3.6.3. Plates shall be embedded within ANSI/TPI 1 tolerances on both faces of the truss at each joint, unless noted otherwise.
- Truss plates shall be centered on the joint unless otherwise specified.

DSB-89 Recommended Design Specification for Temporary Bracing of Metal Plate Connected Wood Trusses, a Truss Plate Institute publication (www.tpinst.org).

NDS: National Design Specification for Wood Construction published by American Forest & Paper Association and American Wood Council.

ESR-2762 Simpson Strong-Tie® AS Truss Plates are covered under ESR-2762 published by the International Code Council Evaluation Service (www.icc-es.org).

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Plate offsets (X, Y):
(None unless indicated below)
Int15(0, -01-05), Int17(0, -01-08),
Int19(0, -01-08), Int1(0-07, -01-08),
Int13(07-07, -01-08)



SIMPSON Component Solutions
True Studio V
2019.1.4.3
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INDIVIDUAL WEB REINFORCEMENT DETAIL

D-WEBREINFORCE

Date: 06/11/2014 Rev: A Pg. 1 of 1

NOTES:

1. This detail provides web reinforcement options that may be used as an alternative to continuous lateral restraint (CLR) when installing CLR's in combination with diagonal bracing is not practical or desired.
2. Refer to the truss design drawing for web lateral restraint requirements. A  on the truss design drawing indicates that continuous lateral restraint is required at the locations shown (either at the midpoint or 1/3-points of the web member). Refer to the tables below for acceptable web reinforcement options that may be used in place of one or two rows of CLR.
3. This detail may not be used to substitute CLR's for T-, L-, I- or scab reinforcements that are specified on the truss design drawing.
4. T-, L-, I- and scab web reinforcements must be the same or better species and grade of the web member as indicated on the truss design drawing.
5. All reinforcements must extend to within 6" of each end of the web member.
6. This detail does not apply to single-ply webs that exceed 14' in length.



1 Row of CLR @
Web Mid-point



2 Rows of CLR's
@ Web 1/3 points

WEB REINFORCEMENT OPTIONS FOR SINGLE-PLY TRUSSES ¹						
Specified Web Member Lateral Restraint (CLR's)	Web Member Size	Acceptable Web Reinforcement Substitutions - Type & Size				Reinforcement-to-Web Connection Requirements
		T-	L-	Scab	I-	
1 Row @ Mid-point	2x4	2x4	2x4	2x4	---	16d gun nails @ 6" on-center
	2x6	2x6	2x6	2x6	---	
	2x8	2x8	2x8	2x8	---	
2 Row @ 1/3-points	2x4	No substitutions allowed			2-2x4	
	2x6				2-2x6	
	2x8				2-2x8	

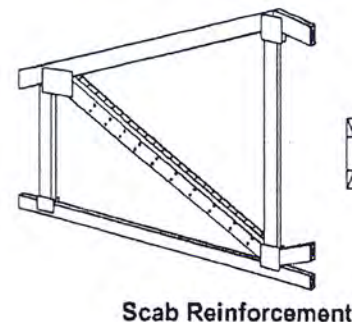
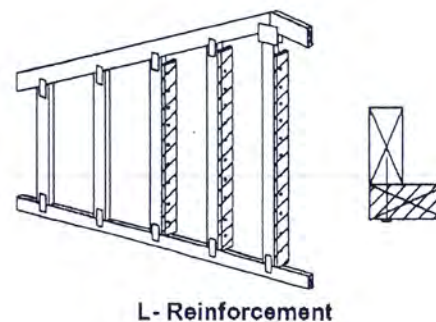
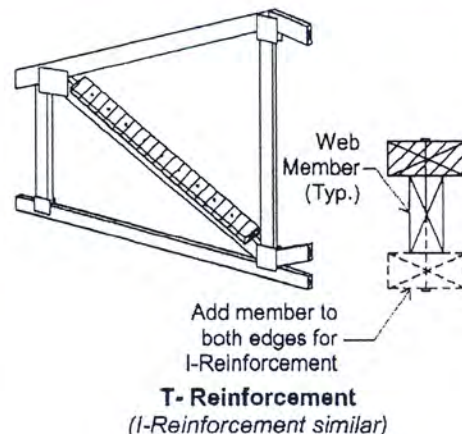
WEB REINFORCEMENT OPTIONS FOR 2-PLY TRUSSES ²						
Specified Web Member Lateral Restraint (CLR's)	Web Member Size	Acceptable Web Reinforcement Substitutions - Type & Size				Reinforcement-to-Web Connection Requirements
		T-	L-	Scab	I-	
1 Row @ Mid-point	2x4	2x4	2x4	---	---	16d gun nails @ 6" on-center
	2x6	2x6	2x6	---	---	
	2x8	2x8	2x8	---	---	
2 Row @ 1/3-points	2x4	No substitutions allowed			2-2x4	
	2x6				2-2x6	
	2x8				2-2x8	

1. The maximum allowable web length for single-ply trusses is 14'.
2. For 2-ply trusses, the reinforcement must be nailed to both plies of the web with the nailing pattern specified in the table.
3. For the scab reinforcement, 2 rows of 10d gun nails @ 6" o.c may be used in place of 16d gun nails for attaching the reinforcement to the web.
4. For I-reinforcement, attach each 2x member to opposite edges of the web using the nailing pattern specified in the table.

Nail Dimension

16d = 3.5" x 0.131"

10d = 3" x 0.120"



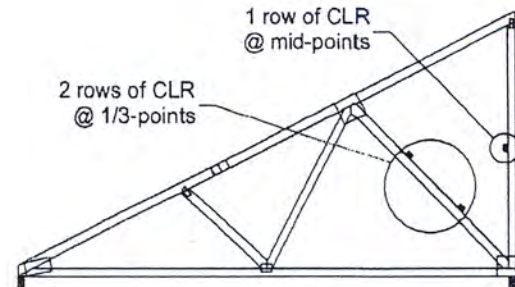


LATERAL RESTRAINT & DIAGONAL BRACING DETAILS FOR WEB MEMBERS

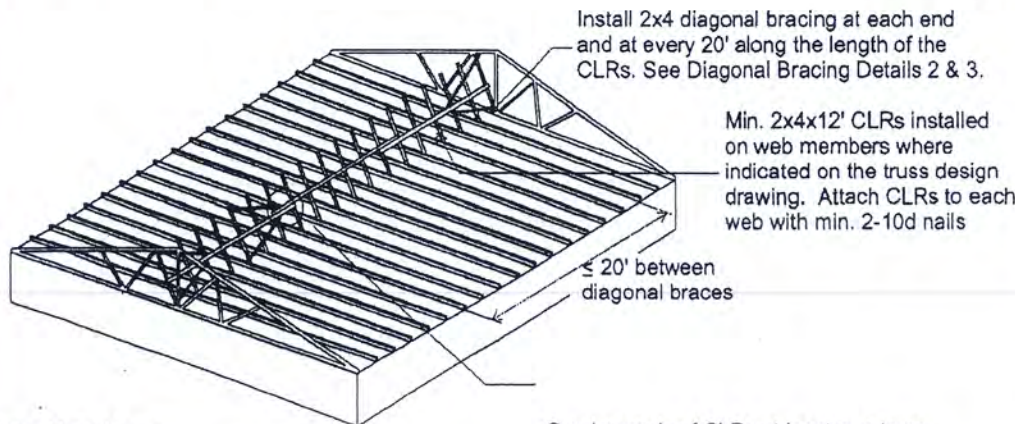
D-WEBCLRBRACE

Date: 06/18/2014 Rev: A Pg. 1 of 1

This detail provides information for laterally restraining and bracing web members to prevent lateral buckling using continuous lateral restraints (CLRs) in combination with diagonal bracing. In addition to the CLRs indicated on the truss design drawing, diagonal bracing must be installed as indicated in this detail and BSCI-B3. See WEBREINFORCE for web reinforcement options that may be used as an alternative to this detail when installing CLRs and diagonal bracing is not practical or desired. Properly attached full-length sheathing satisfies (may replace) any bracing requirements specified for end vertical webs. Refer to the Construction Documents for additional bracing requirements.

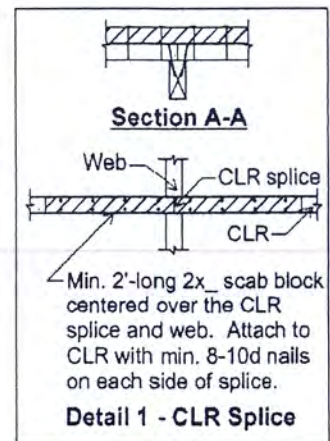


WEB MEMBERS WITH LATERAL RESTRAINT



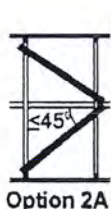
Note: Not all truss members shown for clarity.

Overlap ends of CLRs at least one truss spacing or use splice detail (see Detail 1)

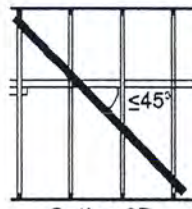


Detail 1 - CLR Splice

For webs with one row of CLRs, diagonal bracing shall be installed using Option 2A or 2B. Attach diagonal braces to each truss with min. 2-10d nails.



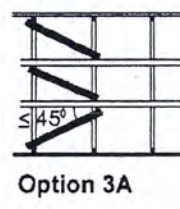
Option 2A



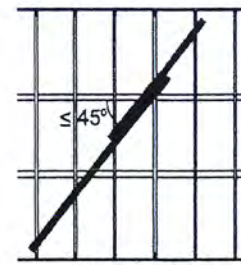
Option 2B

Detail 2 - Diagonal Bracing for 1 Row of CLRs

For webs with 2 rows of CLRs, diagonal bracing shall be installed using Option 3A or 3B. Attach diagonal braces to each truss with min. 2-10d nails.



Option 3A



Option 3B

Detail 3 - Diagonal Bracing for 2 Rows of CLRs

DETAIL LIMITATIONS:

1. Restraint and Bracing Material min. 2x4 stress graded lumber.
2. This detail does not address permanent building stability bracing to resist lateral forces acting on the building.
3. This detail shall not supersede any project-specific truss member permanent bracing design for the roof framing structural system.

Nail Dimensions:

10d = 3" x 0.128"



This detail is to be used only for the application and conditions indicated herein, and its suitability for any particular truss/project shall be verified (by others). This detail is not intended to supersede any project-specific details provided in the Construction Documents. Truss configurations shown are for illustration only. Refer to the truss design drawing(s) accompanying this detail for specific truss design information. Simpson Strong-Tie Inc. is not responsible for any deviations from this design.

Simpson Strong-Tie Company

